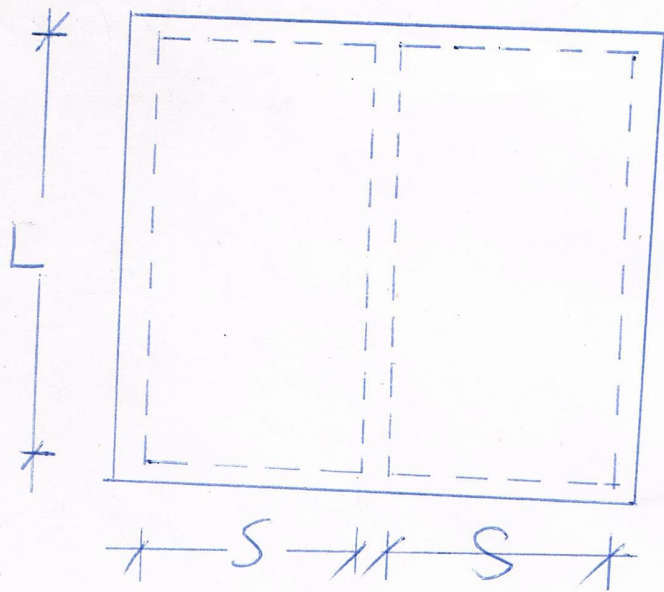
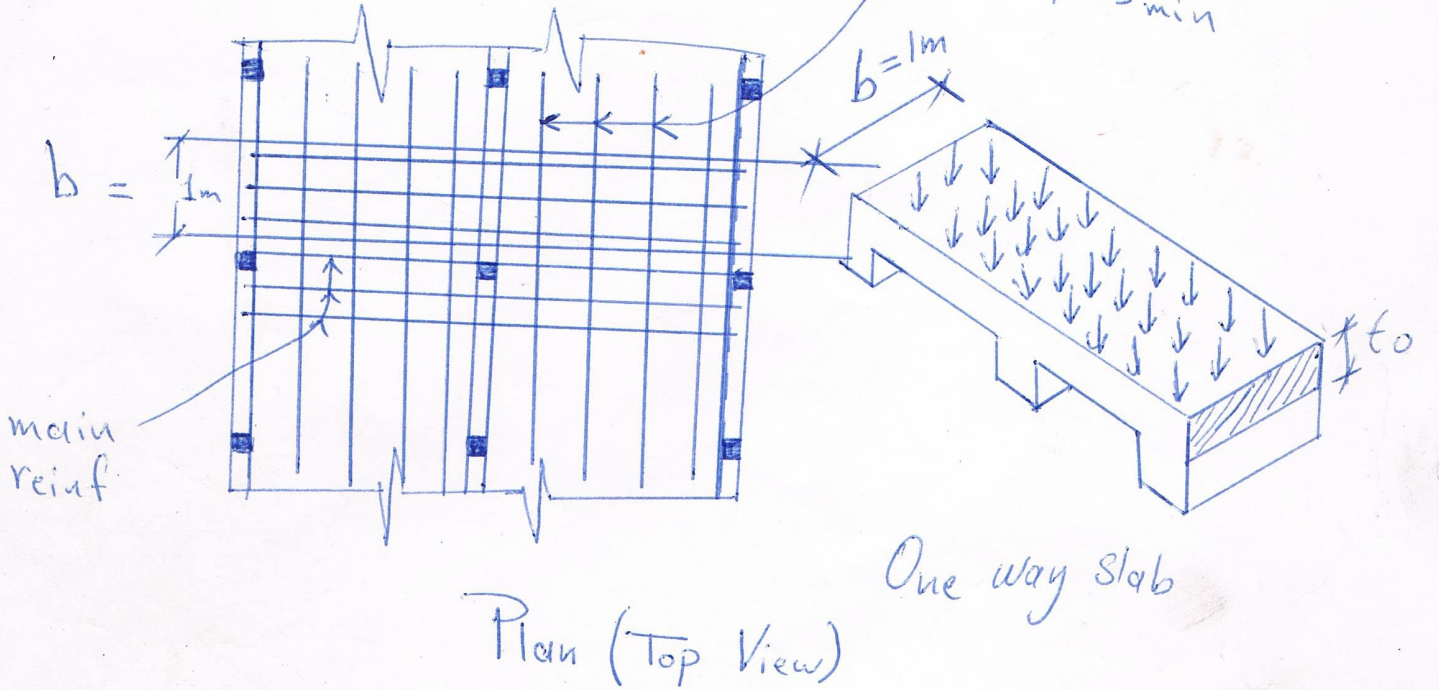


# Reinforced Concrete Slabs

- 1- One-Way Solid Slab
- 2- Two-Way Solid Slab

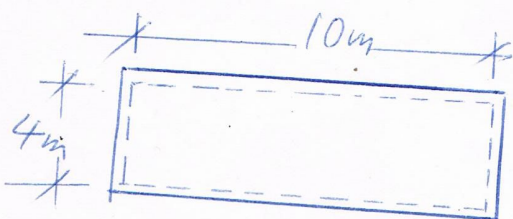


If  $\frac{L}{S} > 2$  One-way slab

$\frac{L}{S} \leq 2$  Two-way slab

$m = \frac{S}{L} < 0.5$  one-way  
 $\geq 0.5$  Two-way

Ex.

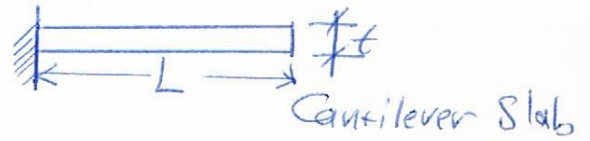


Since  $\frac{10}{4} = 2.5 > 2 \therefore$  one way

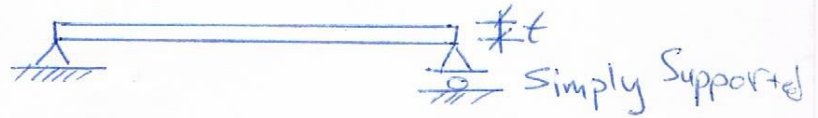
# Slab Thickness

Minimum thickness of one way slab for deflection control

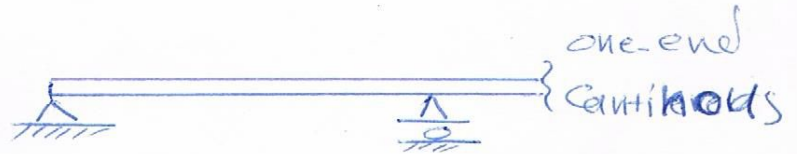
$$t_{min} = \frac{L}{10}$$



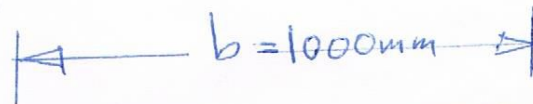
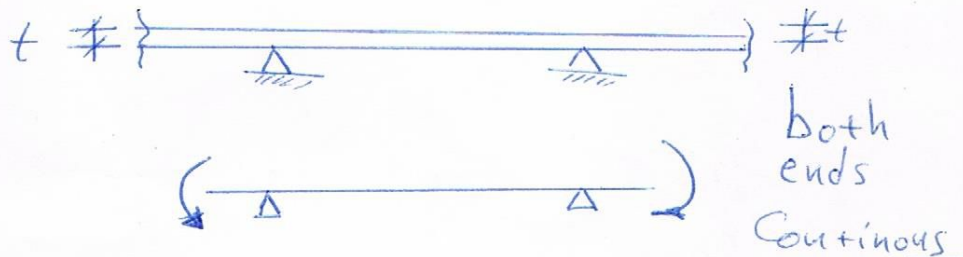
$$t_{min} = \frac{L}{20}$$



$$t_{min} = \frac{L}{24}$$

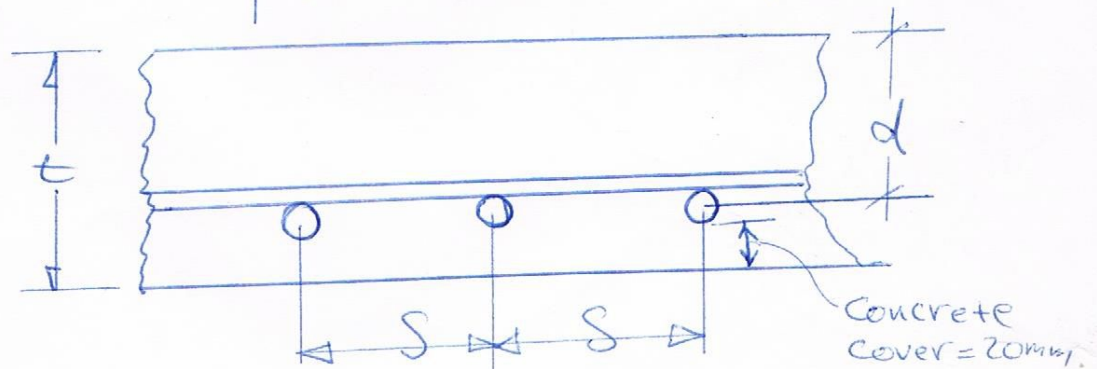


$$t_{min} = \frac{L}{28}$$



$$d = t - 20 - \frac{d_b}{2}$$

$d_b = 10\text{mm}, 12\text{mm}, 16\text{mm}$



S = Spacing between bars

Concrete Cover	
20 mm	slab
40 mm	beam, column
75 mm	foundation

Check the effective depth according to shear requirements

\* The design shear strength ( $\phi V_c$ ) must be equal or greater than design shear force at critical section, if not, the ( $h$ ) must be greater.  
The slab is simply supported,  $f_y = 300 \text{ MPa}$ ,  $f'_c = 20 \text{ MPa}$ .

Ex. Design the slab (Roof) showing in the Fig. below:-

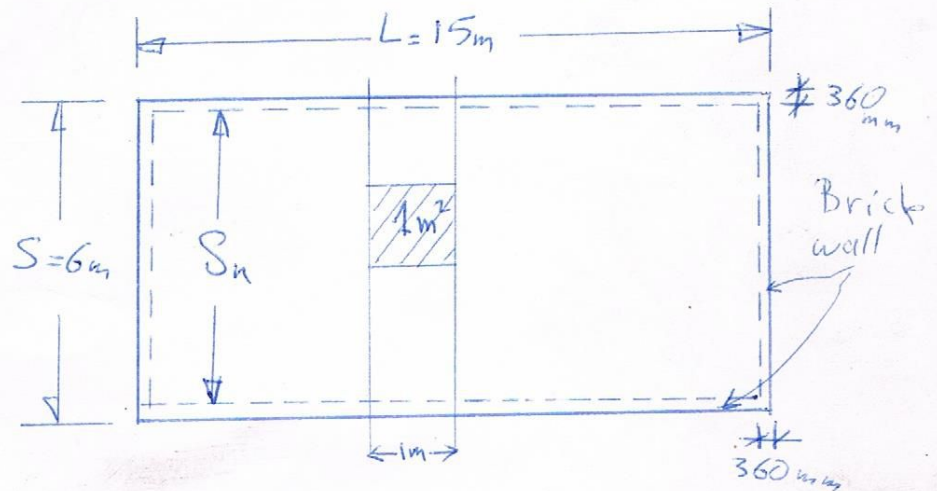
WL =  $1.5 \text{ kN/m}^2$ , Tiles =  $1.0 \text{ kN/m}^2$ , Earth filling =  $2 \text{ kN/m}^2$   
assuming the average thickness of earth filling =  $100 \text{ mm}$

Solution:-

$$* \frac{L}{S} = \frac{(15 - 2 \times 0.36)}{(6 - 2 \times 0.36)}$$

$$= 2.70 > 2$$

$\therefore$  The slab is one way slab.



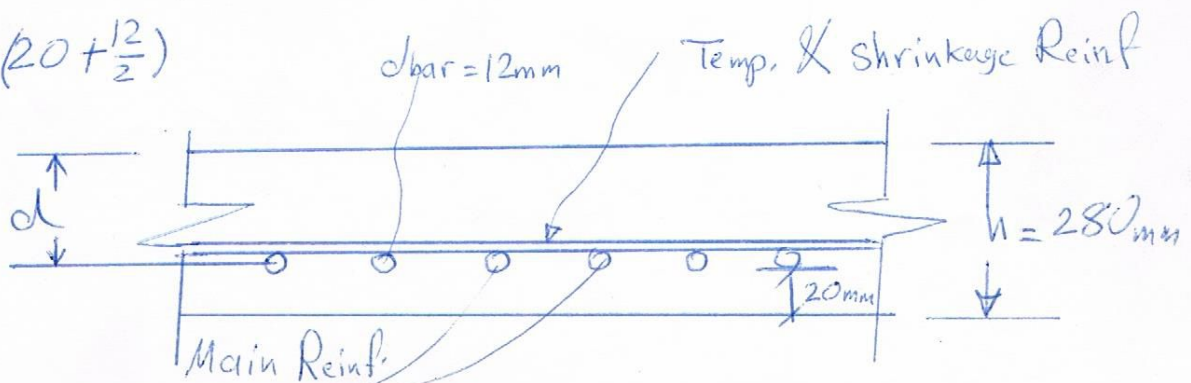
$$* h = \frac{6 - 2 \times 0.36}{20} \times 1000 = 264 \text{ mm} \quad \text{Use } h = 280 \text{ mm}$$

$$* W_{\text{DL}} = W_{\text{tiles}} + W_{\text{earth}} + W_{\text{self}} = 1 + 2 + 24 \times 1 \times 1 \times 0.28 = 9.72 \text{ kN/m}^2$$

$$W_u = 1.6 \times 1.5 + 1.2(9.72) = \sim 14.1 \text{ kN/m}^2$$

$$d = 280 - (20 + \frac{12}{2})$$

$$d = 254 \text{ mm}$$



### \* Checking for shear

$$V_{us} = \frac{14.1 \times 5.28}{2} = 37.22 \text{ kN}$$

$$V_{ud} = V_{us} - W_{ud} = 37.22 - 14.1 \times 0.254 = 33.643 \text{ kN}$$

$$\phi V_c = 0.75 \times \frac{1}{6} \sqrt{f_c'} \times b \times d = 0.75 \times \frac{1}{6} \sqrt{20 \times 1000 \times 254 \times 10^3}$$

$$\phi V_c = 142 \text{ kN} > V_{ud} = 33.64 \text{ kN}$$

$\therefore$  The thickness is adequate for shear.

### \* Bending Moment :-

$$M_{u_{\max}} = \frac{W_u S_u^2}{8} = \frac{14.1 \times (5.28)^2}{8} = 49.14 \text{ kNm}$$

$$\rho_{\max} = 0.0206$$

$$M_u = \phi \rho f_y b d^2 \left(1 - \frac{0.59 \rho f_y}{f_c}\right)$$

$$49.14 \times 10^6 = 0.9 \times 300 \times \rho \times 1000 \times 254^2 \left(1 - \frac{0.59 \times \rho \times 300}{20}\right)$$

$$49.14 \times 10^6 = 1.742 \times 10^{10} \rho - 1.542 \times 10^{11} \rho^2$$

$$\rho^2 - 0.113 \rho + 0.00031 = 0$$

$$\rho = \frac{-0.113 \pm \sqrt{0.113^2 - 4 \times 1 \times 0.00031}}{2}$$

$$\rho = 0.0028 < \rho_{\max} = 0.0206$$

$$A_{s_{\min}} = 0.002 b h = 0.002 \times 1000 \times 280 = 560 \text{ mm}^2/\text{m}$$

$$A_s = \rho b d = 0.0028 \times 1000 \times 254 = 711.2 \text{ mm}^2/\text{m} > A_{s_{\min}}$$

$$\rho_t = 0.018 > \rho \quad \therefore \phi = 0.9 \text{ OK}$$

$$A_{s\text{bar}} = \frac{\pi}{4} \times 12^2 = 113 \text{ mm}^2, S_{\text{max}} \leq \begin{cases} 450 \text{ mm} \checkmark \\ 3h = 3 \times 280 = 840 \text{ mm} \end{cases}$$

$$S = \frac{1000}{712/113} = 158.7 \text{ mm} \text{ c/c} \quad \text{Use } S = 150 \text{ mm} \text{ c/c} < 450 \text{ mm}$$

$A_s$  for Temperature & shrinkage

$$A_s = A_{s\text{min}} = 560 \text{ mm}^2/\text{m}$$

$$S = \frac{1000}{560/113} = 201.8 \quad S_{\text{max}} \leq \begin{cases} 5h = 1400 \text{ mm} \\ 450 \text{ mm} \checkmark \end{cases}$$

$$\therefore \text{Use } S = 200 \text{ mm} \text{ c/c}$$

نقره ~~از~~ نصف در التسلح

The B.M. value in any Point at distance =  $x$

$$\text{is } M_x = \frac{w s_n}{2} * X - \frac{w X^2}{2}$$

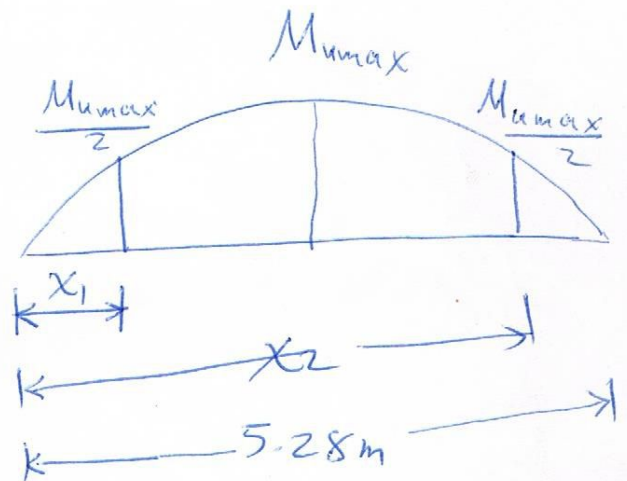
$$\left( \frac{49.14}{2} \right) = \frac{14.1 * 5.28}{2} * X - \frac{14.1 X^2}{2}$$

$$24.57 = 37.224X - 7.05X^2$$

$$X^2 - 5.28X + 3.485 = 0$$

$$X = \frac{5.28 \pm \sqrt{5.28^2 - 4 * 1 * 3.85}}{2}$$

$$X_1 = \frac{1.748}{2} = 0.874 \text{ m}, \quad X_2 = \frac{8.812}{2} = 4.41 \text{ m}$$



نقاط العود الحقيقية .

$$x_{1 \text{ Real}} = X_1 - \frac{d}{2} = 0.874 - \frac{0.254}{2} = 0.747 \text{ m}$$

$$x_{2 \text{ Real}} = X_2 + \frac{d}{2} = 4.41 + \frac{0.254}{2} = 4.537 \text{ m}$$

