

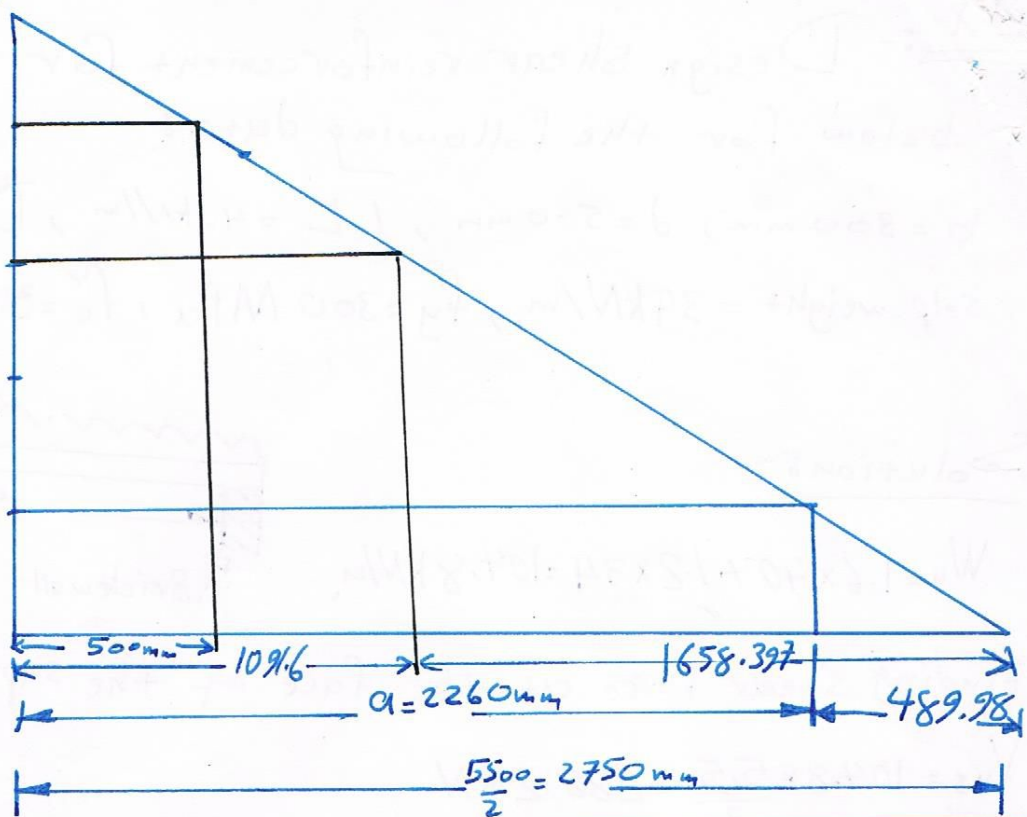
$$V_{us} = 288.2 \text{ kN}$$

$$V_{ud} = 235.8 \text{ kN}$$

$$V_{u \min} = 173.8$$

$$\phi V_c = 102.698 \text{ kN}$$

$$\phi V_c / 2 = 51.35 \text{ kN}$$



$$* \frac{\phi V_c}{2} = \frac{102.698}{2} = 51.35 \text{ kN}$$

$$\begin{aligned} &\because \phi V_s < 4 \phi V_c \quad \left\{ \begin{array}{l} d/2 = 250 \text{ mm} \checkmark \\ 600 \text{ mm} \end{array} \right. \\ * \therefore S_{\max} &\leq \left\{ \begin{array}{l} 600 \text{ mm} \\ \frac{3A_v f_y}{b_w} = \frac{3 \times 2 \times 79 \times 300}{300} = 474 \text{ mm} \\ \frac{16A_v f_y}{\sqrt{f_c} b_w} = \frac{16 \times 2 \times 79 \times 300}{\sqrt{30} \times 300} = 461 \text{ mm} \end{array} \right. \\ \therefore \text{Use } S_{\max} &= 250 \text{ mm} \end{aligned}$$

* Find spacing of reinforcement at critical section.

$$S_o = \frac{A_v f_y d}{V_s} \Rightarrow \text{use } \phi 10 \text{ mm for stirrups.}$$

$$\therefore A_v = 2 * \frac{\pi}{4} * 10^2 = 157.0 \text{ mm}^2$$

$$S_o = \frac{157 * 300 * 500}{177.47 * 10^{-3}} = 132.7 \text{ mm} < S_{\max} = 250 \text{ mm}$$

$$\therefore \text{Use } S_o = 130 \text{ mm } \frac{c}{c}$$

* Finding the distance in which there is no need for reinf.:

$$\frac{x}{51.35} = \frac{2750}{288.2}$$

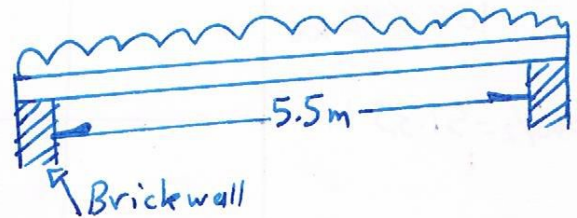
$$\Rightarrow x = 489.98 \text{ mm}$$

$$a = 2750 - 489.98 = 2260 \text{ mm}$$

E.x.:- Design shear reinforcement for the beam shown below for the following data:-

$b = 300 \text{ mm}$, $d = 500 \text{ mm}$, $LL = 40 \text{ kN/m}$, DL including self weight $= 34 \text{ kN/m}$, $f_y = 300 \text{ MPa}$, $f_c' = 30 \text{ MPa}$.

Solution:-



* $W_u = 1.6 \times 40 + 1.2 \times 34 = 104.8 \text{ kN/m}$

* finding shear force at the face of the support-

$$V_{us} = 104.8 \times \frac{5.5}{2} = 288.2 \text{ kN}$$

* Finding shear force at critical section.

$$V_{ud} = V_{us} - W_u d = 288.2 - 0.5 \times 104.8 = 235.8 \text{ kN}$$

* $\phi V_c = 0.75 \left(\frac{1}{6} \sqrt{f_c'} \times b \times d \right)$

$$= 0.75 \left(\frac{1}{6} \sqrt{30} \times 300 \times 500 \right) \times 10^{-3} = 102.698 \text{ kN}$$

Check if there is need for shear reinforcement

$$V_{ud} = 235.8 > \phi V_c = 102.698 \quad \therefore \text{there is a need for shear reinforcement}$$

* $\phi V_s = V_{ud} - \phi V_c = 235.8 - 102.698 = 133.1 \text{ kN}$

$$V_s = \frac{133.1}{\phi} = \frac{133.1}{0.75} = 177.47 \text{ kN}$$

* $4 \phi V_c = 4 \times 102.698 = 410.792 \text{ kN}$

$$\therefore \phi V_s < 4 \phi V_c$$

$$133.1 \text{ kN} < 410.792 \text{ kN}$$

\therefore The section is adequate for shear reinforcement.

- We can find the distance (a) by other method.

$$V_{us} - W_u * a = \phi V_c / 2$$

$$288.2 - 104.8 * a = 51.35 \Rightarrow a = \frac{51.35 - 288.2}{-104.8}$$

$$\therefore a = 2.260 \text{ m}$$

* Determine the distance, which it is after reinforced by minimum reinforcement (shear reinforcement).

$$\phi V_{s_{min}} = \frac{\phi A_v f_y d}{S_{max}} = \frac{0.75 * 157 * 300 * 500 * 10^{-3}}{250} = 70.650 \text{ kN}$$

$$V_{u_{min}} = \phi V_{s_{min}} + \phi V_c = 71.1 + 102.7 = 173.8 \text{ kN}$$

$$V_{us} - W_{ub} = 173.8$$

$$288.2 - 104.8 * b = 173.8 \Rightarrow b = \frac{173.8 - 288.2}{-104.8}$$

$$b = 1.0916 \text{ m}$$

$$\text{or } \frac{x}{173.8} = \frac{2750}{288.2} \Rightarrow x = 1658.397 \Rightarrow x_1 = 2750 - 1658.397$$
$$\Rightarrow x_1 = 1091.6 \text{ mm}$$

* Distribution of shear reinforcement along the beam

a- Put the first stirrups at a distance equal to $\frac{S_o}{2} = \frac{130}{2} = 65 \text{ mm}$

\(\therefore\) Put the first stirrups at a distance equal to 60 mm from the face of the support.

b- Number of other stirrups (130 mm)

$$n = \frac{1092 - 60}{130} = 7.938$$

Use 8 $\phi 10$ stirrup @ 130 mm

c - S_0 , the distance from the face of the support which reinforced for shear until now equal to:

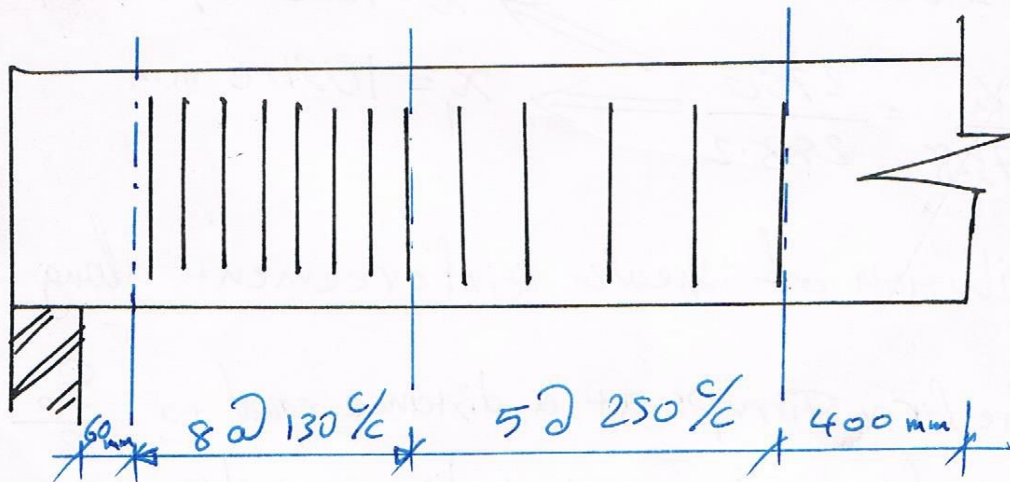
$$60 + 8 \times 130 = 1100 \text{ mm}$$

$$\therefore \text{No of stirrups of } 250 \text{ mm } \phi = \frac{2260 - 1100}{250}$$
$$= 4.64$$

\therefore use $5 \phi 10 \text{ mm}$ stirrups @ $250 \text{ mm } \phi$

So, the space which reinforced to shear equal to $1100 + 5(250) = 2350 \text{ mm}$

So, the region which not reinforced for shear is equal to $2750 - 2350 = 400 \text{ mm}$



or from equilibrium

$$V_{us} = W_u a = \phi V_c / 2$$

$$288.2 - 104.8 a = 51.35 \text{ kN} \Rightarrow a = 2260 \text{ mm}$$

finding the distance which after this distance

the shear reinforcement in minimum magnitude

$$\begin{aligned} \phi V_{s \min} &= \frac{\phi A_v f_y d}{s_{\max}} = \frac{0.75 \times 2 \times 79 \times 300 \times 500 \times 10^{-3}}{250} \\ &= 71.1 \text{ kN} \end{aligned}$$

$$V_{u \min} = \phi V_{s \min} + \phi V_c = 71.1 + 102.7 = 173.8 \text{ kN}$$

$$V_{us} - W_u b = 288.2 - 104.8 b = 173.8 \Rightarrow b = 1092 \text{ mm}$$

the distance between critical section & the point of minimum shear reinf. is small

$$e = b - 500 = 1092 - 500 = 592 \text{ mm}$$

So use the same shear reinf. for critical section